### 12.005 Lecture Notes 15

## Elasticity

So far:
Stress $\rightarrow$ angle of repose vs accretionary wedge
Strain $\rightarrow$ reaction to stress $\rightarrow$ but how?

## Constitutive relations

$$
\tau_{i j}=\tau_{i j}\left(\varepsilon_{k l}\right) ; \quad \varepsilon_{i j}=\varepsilon_{i j}\left(\tau_{k l}\right)
$$

For example,
Elasticity
Isotropic
Anisotropic
Viscous flow
Isotropic
Anisotropic
Power law creep
Viscoelasticity

Trade offs:

| simplicity | $\leftrightarrow$ | realism |
| :---: | :---: | :---: |
| constant |  | variable |
| isotropic |  | anisotropic |
| elastic, viscous |  | viscoelastic |
| history <br> independent |  | history dependent |

## Tensors

Most physical quantities that are important in continuum mechanics like temperature, force, and stress can be represented by a tensor. Temperature can be specified by stating a single numerical value called a scalar and is called a zeroth-order tensor. A force, however, must be specified by stating both a magnitude and direction. It is an example of a first-order tensor. Specifying a stress is even more complicated and requires stating a magnitude and two directions - the direction of a force vector and the direction of the normal vector to the plane on which the force acts. Stresses are represented by secondorder tensors.

Tensors are quantities independent of coordinate system.


Figure 15.1
Figure by MIT OCW.

$$
\alpha_{i j}=\cos \phi_{i j}
$$

where $\phi_{i j}$ is the angle of primed to original.

$$
\begin{aligned}
& x_{i}{ }^{\prime}=\alpha_{i j} x_{j} \\
& x_{i}=\alpha_{j i} x_{j}{ }^{\prime} \\
& \alpha_{i j}=\frac{\partial x_{i}{ }^{\prime}}{\partial x_{j}}=\frac{\partial x_{j}}{\partial x_{i}{ }^{\prime}}
\end{aligned}
$$

Tensors:
a. $\quad 0^{\text {th }}$ order (scalar) - quantity dependent only on position
b. $1^{\text {st }} \operatorname{order}\left(3^{1}\right.$ components $) \quad A_{i}{ }^{\prime}=\alpha_{i j} A_{j}$
c. $2^{\text {nd }} \operatorname{order}\left(3^{2}=9\right.$ components $) \quad A_{i j}{ }^{\prime}=\alpha_{i s} \alpha_{j k} A_{s k}$
d. $3^{\text {rd }} \operatorname{order}\left(3^{3}=27\right.$ components $) \quad A_{i j k}{ }^{\prime}=\alpha_{i s} \alpha_{j t} \alpha_{k p} A_{s t p}$
e. $4^{\text {th }} \operatorname{order}\left(3^{4}=81\right.$ components $) \quad A_{i j k l}{ }^{\prime}=\alpha_{i s} \alpha_{j t} \alpha_{k p} \alpha_{l q} A_{s t p q}$

## Linear Elastic Solid

$\tau_{i j}=c_{i j k l} e_{k l}$
$c_{i j k l}$ is elastic modulus tensor
$c_{i j k l}=\underline{\text { constants }}(\neq$ history, $\neq$ displacement, $\neq$ time $)$
$3^{4}=81$ components
$\tau_{i j}=\tau_{j i}, e_{i j}=e_{j i} \rightarrow$ cuts to 36

Strain Energy
$U=\frac{1}{2} \tau_{i j} e_{i j} \Rightarrow \tau_{i j}=\frac{\partial U}{\partial e_{i j}}$

Write in terms of powers of $e_{i j}$
$U=\alpha+\beta_{i j} e_{i j}+\gamma_{i j k l} e_{i j} e_{k l}$
0 (to avoid spontaneous expansion, contraction)
$\frac{\partial U}{\partial e_{i j}}=\left(\gamma_{i j k l}+\gamma_{k l i j}\right) e_{k l}$
$\downarrow$

$$
c_{i j k l}=c_{k l i j} \Rightarrow 21 \text { individual components. }
$$

21 components - data fitting - ugh!
Lots of available information, but lots of hard work.
Reality - 21 components (triclinic)
$\downarrow$
Simplicity - derive in homework
Isotropic
$c_{i j k l}=\lambda \delta_{i j} \delta_{k l}+\mu\left(\delta_{i k} \delta_{j l}+\delta_{i l} \delta_{j k}\right)$
$\lambda$ and $\mu$ are Lame parameters.
$\tau_{i j}=\lambda e_{k k} \delta_{i j}+2 \mu e_{i j}$
Can also express

$$
\begin{gathered}
e_{i j}=S_{i j k l} \tau_{k l} \\
\\
\uparrow
\end{gathered}
$$

Compliance tensor
Aside

$$
\begin{gathered}
e_{11}=S_{1132} \tau_{32} \\
\uparrow
\end{gathered}
$$

triclinic, trigonal


Figure 15.2
Figure by MIT OCW.

Sheer leads to lengthening.
Isotropic - can relate $c_{i j k l}$ and $S_{i j k l}$ directly
$\tau_{i i}=\lambda e_{k k} \delta_{i i}+2 \mu e_{i i}=(3 \lambda+2 \mu) e_{i i}$
$2 \mu e_{i j}=\tau_{i j}-\frac{\lambda \delta_{i j}}{2 \mu+3 \lambda} \tau_{k k}$

## Conventional moduli:

1. Hydrostatic comp.


Figure 15.3
Figure by MIT OCW.

$$
\begin{aligned}
\tau_{i j} & =-p \delta_{i j} \\
\tau_{i i} & =-p \delta_{i i}=3 \lambda e_{k k}+2 \mu e_{i i} \\
& =-3 p=(3 \lambda+2 \mu) e_{i i} \\
-\frac{p}{e_{i i}} & =-\frac{V P}{\Delta V} \equiv K
\end{aligned}
$$

where $K=\lambda+2 / 3 \mu$ is bulk modulus.
2. Uniaxial stress


Figure by MIT OCW.

$$
\tau_{11}=T
$$

other $\tau_{i j}=0$

$$
\begin{aligned}
& 2 \mu e_{1}=T-\frac{\lambda}{2 \mu+3 \lambda} T \\
& \frac{T}{e_{1}} \equiv E(\text { sometimes } Y)
\end{aligned}
$$

where $E=\frac{\mu(2 \mu+3 \lambda)}{\mu+\lambda}$ is Young's modulus
Hook's law:

$$
\begin{aligned}
& T=E e \\
& \frac{e_{22}}{e_{11}}=\frac{e_{33}}{e_{11}} \equiv-v \quad \text { This is called Poisson's ratio. } \\
& 2 \mu e_{22}=-\frac{\lambda}{2 \mu+3 \lambda} \tau_{11} \Rightarrow v=\frac{\lambda}{2(\mu+\lambda)} \\
& \theta=e_{11}+e_{22}+e_{33}=e_{11}(1-2 v) \\
& \text { fluid: } \mu \rightarrow 0 \Rightarrow v \rightarrow \frac{1}{2}
\end{aligned}
$$

$$
\text { most material: } v=0.2-0.3
$$

$$
v=\frac{1}{4} \Rightarrow \lambda=\mu \quad \text { It is Poisson solid. }
$$

steel: $v ; 0.3-0.33$
seismically measured

$$
v_{p}=\sqrt{\frac{\lambda+2 \mu}{\rho}} \quad v_{s}=\sqrt{\frac{\mu}{\rho}}
$$

$$
\text { compare } v_{p}, v_{s} \rightarrow v \rightarrow \text { discriminate rock types }
$$

3. Simple shear

$$
\begin{aligned}
& \\
& \tau_{12}= \\
& \tau_{12}= 2 \mu e_{12}=2 G e_{12}
\end{aligned}
$$

where $G$ is shear modulus.

Note: Among $\lambda, \mu, K, v, E, G$ only two are independent.

Useful forms:

$$
\begin{aligned}
& \tau_{11}=(\lambda+2 \mu) e_{11}+\lambda e_{22}+\lambda e_{33} \\
& \tau_{22}=\lambda e_{11}+(\lambda+2 \mu) e_{22}+\lambda e_{33} \\
& \tau_{33}=\lambda e_{11}+\lambda e_{22}+(\lambda+2 \mu) e_{33}
\end{aligned}
$$

or

$$
\begin{aligned}
& e_{11}=\frac{\tau_{11}}{E}-\frac{v \tau_{22}}{E}-\frac{v \tau_{33}}{E} \\
& e_{22}=-\frac{v \tau_{11}}{E}+\frac{\tau_{22}}{E}-\frac{v \tau_{33}}{E} \\
& e_{33}=-\frac{v \tau_{11}}{E}-\frac{v \tau_{22}}{E}+\frac{\tau_{33}}{E}
\end{aligned}
$$

